#### LECTURE NOTE

ON

### LAND SURVEY-II (TH.1)

6<sup>TH</sup> SEMESTER IN CIVIL ENGG.



#### PREPARED BY

#### Er. PRIYABRATA TRIPATHY

(LECTURER)

DEPARTMENT OF CIVIL ENGG.

G.I.E.T (POLYTECHNIC),JAGATPUR,CUTTACK,ODISHA > It is a boranch of sureversing in which horizontal and veretical distances are determined by taking angulare observation with an instrument known as tachemeter.

> This surveying is adopted in reough and difficult tererain where direct levelling and chairing are either

not possible on verey ledéous.

-) This scereieziona is verez reapid and a recosomable contoure map can be proposed fore investigation cooreks within a shoot time on the basis of such survey,

objectives: Tacheometry is the prepareation of contour romps ore plans requiring both the horizontal as well as vertical control. Also, on surveys of higher accuracy, it provides a check on distances measured with the

USES "

- 1. Preparation of topographic map coherer both horizontal and vertical distances are required to be masured.
- 2. Surevery work in difficult bererain where direct methods of measureements aree oconvenient;
- 3. Recomaissance surevey for highways and realways, etc.
- 4. Establishment of secondary control points.

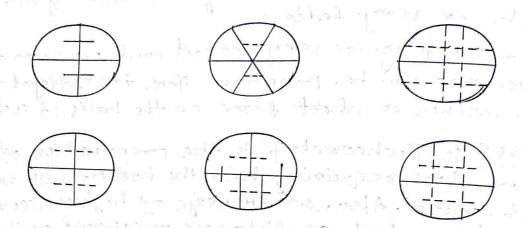
# Tacheometroec Sureveying Instrouments.

Tacheometric surveying is done with the help of Tacheometer and Stadia Rod.

- 1. Tacheomekere
- 2. Stadia Rod
- 3. Amallatic lens.
- 1. Tacheometere: A treaseit theodolite fixed with special stadia diaphoagon is Known as tacheometen. It is the main instrument of tacheometric surveying. 918 telescope contains two horizontal hairs called Stadia haires in addition to the regular crosshain

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The stadia haires area equidistance From (3) the contral cross-haires and they aree opecially leverned as steedia lives one stadéa webs. The Coronosos trypes of stadia diaphragons area shown below:



- 2. Stadia Rod: Fore small distances (rup to 100 meleves) a level staff may be used fore techeometréc surveying Bret fore greatere distances stadia read is needed! Stadia read is of one piece having 3 to 5 melerelegation The smallest subdivision is reseally 5 mm.
- 3. An allatic less :- et is an additional less used in the instrument. It is a special lens which is placed between the object glass and the eyepèece of the telescope resordere to ellionisale the additive constant. (f+d). This is done to make the expression for the distance between instrument station and staff position more simplified. The lens is only provided in an external focusing belescope but not in the rinteresal focusing.

Features of Tacheometer: -

- I' Multiplying constant value of 100 and additive
- d. Axial horizontal line should be exactly midway between the other two lines.
- 3. Telescope should be touly anorllatic.

4. Telescope should be powereful, having a magnification

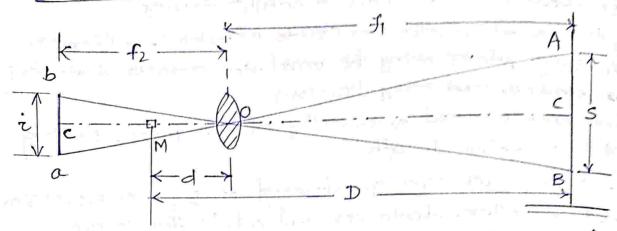
- 5. Aperefuree of the objective should be 35 to 45mm in diametere to have a bright image.
- 6. For small distances (repto 100 meters), oredinatez levelling statt may be resed, fore greatere distances a stadia read may be resed,
- 7. A stadia read is usually of one piece, having 3 to 5 mater length.
- 8. A stadia read greaterated in 5 mm (ci.e, 0.005 m) fore smallere destances and while fore longere distances, the rood may be greaterated in 1 cm (i.e, 0.01m)

## Preencèple 07 stadia method:

The stadia method is based on the prioreiple that the realise of the perpendiculare to the base is constant in similare isosceles tolangles. Two reals OA and OB be equally inclined to central may oc. Let A2B2, AtB1 and AB be staff intercepts.

From the figure we can write,  $\frac{OCZ}{A2B2} = \frac{OCG}{A1BI} = \frac{OC}{AB} = \frac{1}{2} = \frac$ 

- i= interval between the stadia hains (stadia interval)
  This constant K entirety depends repor the oragonitude
  of the angle B.
- -> 90 actual practice, observations may be made with either boxizontal line of sight are with inclined line of sight.
- -> go laker case the staff may be kept either vertilically ore normal to the line of sight.
- -) firest the distance elevation formula fore the horizontal sights should be dereived.



Considere the figures, in which o is the optical confree of the objective of an exterenal focusing telescope.

Let A, C and B area the points cut by the three lines of sight correctsponding to three wires. b, c and a area the top, ascial and bottom hairs of diaphroagon.

ab = i = intereval between the stadia baires.

AB = s = stass Enterecept

f = focal length of the objective.

It = hosizontal distance of the staff from the optical centre of the objective

\$2 = horizontal distance of the creass-wirels from 0.

d = distance of vertical axis of instrument from 0

D = horizontal distance of staff from vertical axis

of the instance of.

M = Centree of the instrument, converponding to the vertical axis.

Since the reags BOb and ADa pass through the optical centre, they are straight to that ADB and aDB area similare, Hence,

Again since, formal de aree conjuage focal distance we have From les foremula,

$$\frac{1}{f} = \frac{1}{f_2} = \frac{1}{f_1}$$

(5)

Multiplifying throughout by  $ff_1$ , we get  $f_1 = f_1 f_1 f_2 f_3 f_4 f_4$ Substituting the values of  $f_1 = f_2 f_3 f_4 f_4$ we get,  $f_1 = f_2 f_3 f_4 f_4$ Horizontal distance between the axis and the staff is  $D = f_1 + f_4$ 

 $D = \frac{f}{2}S + (f+d) = K \cdot S + C$ , this equal Earlie Known as the distance equation.

The constant  $K = \frac{f}{2}$  is known as the multiplying constant one stadia interval factors and the constant (f+d) = C is known as the additive constant of the instanment.

## Deteremination of constant Kand C:

1st Method: - This the laboreatorey method. In this method, the additive constant c = CF+d) is measured from the Enstowment while the multiplying constant K is computed from field observation.

10 focus the instancent to a distant object and measure along the telescope the distance between the objective and creass-baires.

2. The distance of between the instrument assist and the objective is variable in the case of external focusing beliescope, being greeater fore short sights and smaller fore long sights. It should, therefore be measured fore average sight. Thus the additive constant Cf+d) is known.

3. To colculate the most Eptying constant K, measure a Known destance D<sub>1</sub> and take the intercept  $S_1$  on the staff kept at that point, the line of sight being boxizontal. Using the equation,  $D_1 = KS_1 + C$  ore  $K = \frac{D_1 - C}{S}$ 

Fore average values staff intercepts, S2, S3, etc. Can be oneasureed corresponding to distance D2, D3, etc can mean value can be calculated.

and Method: - In this method, both the constants cerce deteremined by field observations as under:

1. Measuree a line about 2000 long, on fairely leveled greound and dreive pegs at some interval. Say 50 moters.

a keep the staff on the pegs and observe the corereesponding staff interecepts with honizontal sight

3. Knowing the values of D and s fore different points, a number of simultaneous equation con be substituting the values of D and S in equation D = Ks + C. The simultaneous solution of successive pairs will give the values of K and C, and the average of these can be found.

fore example, if SI is the staff interecept coreresponding to distance D1 and S2 coreresponding to De we have,

D1 = Ks, +C - 1 and D2 = Ks2+C - 1 Subtracting 1) from 11 we get,

$$k = \frac{D_2 - D_1}{S_2 - S_1} - \boxed{2}$$

Substitutions the value of Kin (1), we get

$$C = D_1 - \frac{D_2 - D_1}{S_2 - S_1} S_1 = \frac{D_1 S_2 - D_2 S_1}{S_2 - S_1} = \frac{2}{S_2 - S_1}$$
Thus, easier 1)

Thus equection @ and @ give the values of k and C.

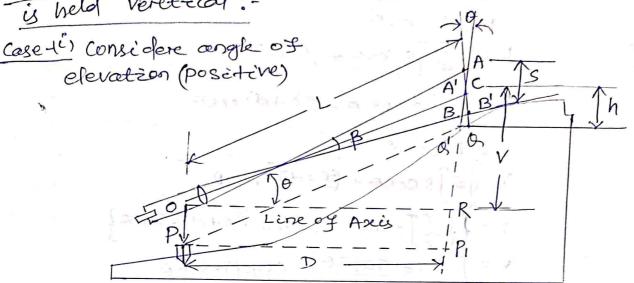
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DIEKSITC OC. K=

P

Expression of Horeizontal distance and verelical Elevation, ref line of sight is inclined and staff is held veretical:



(Line of sight is inclined and staff held veretical)

Let P = Instaument station; Q = Staff station

M= Position of instruments axis;

0 = optical centure of the those hairs.

S=AB= Stass Entercept; == Stadza Entereval

0 = Forchination of line of sight from the hooizental

L=Length oc measured along the line of sight.

D = OR = Hosizontal distance between the instrument and staff.

V- Veretical intercept at a between the line of sight and the hosizontal line.

h = height of the Enstoument;

re = contral hair relading

B = Angle between two extreme rays commes ponding to stadia hairs.

Draw a line A'CB' normal to line of sight oc. Angle AA'C = 90° + B/z, being the extercion angle of the A COA' Sinsilarly, from A COB', angle OB'C = angleBB'C=90°-P/2 Since B/z is very small Cits value = 17'11" for k=100)

LAA'C and LBB'C = 90°

From A ACA!, A'C = ACCOSO OFE A'B' = ABCOSO = SCOSO Since line A'B' is perpendicular to line of sight OC, equation D = ks + C is derectly applicable.

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We have, 
$$S' = AC \cos \theta + B \cos \theta$$
  
 $S' = CAC + BC) \cos \theta$   
 $S' = S \cos \theta$   
 $D = L \cos \theta$   
 $L = \begin{bmatrix} f \\ 2 \end{bmatrix} S \cos \theta + (f+d)$   
 $D = \begin{bmatrix} f \\ 2 \end{bmatrix} S \cos \theta + (f+d) \cos \theta$   
 $V = \begin{bmatrix} f \\ 2 \end{bmatrix} S \cos \theta + (f+d) \sin \theta$   
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\* 95 line of sight is inclined experients = H.I. of instrument assist +V-h

\* If line of sight is inclined downwareds = H.I. of instrument asis -V-h.

Case-cii) Considere angle of deposssion (negative)

$$L = \left(\frac{1}{2}\right) S \cos \theta + C + C + d$$

$$V = \left(\frac{f}{2}\right) \times \frac{S \times 20}{2} + C + C + d) \otimes in\theta$$

R.L of staff station = RL of line of collimation-v-h